GENERAL NOTES

GENERAL

MEET SPECIAL CONDITIONS.

- 1.1 GENERAL NOTES AND TYPICAL STRUCTURAL DETAILS SHALL APPLY TO ALL DRAWINGS UNLESS OTHERWISE SHOWN OR NOTED.
- 1.2 FEATURES OF CONSTRUCTION SHOWN ARE TYPICAL AND SHALL APPLY GENERALLY THROUGHOUT FOR SIMILAR CONDITIONS. MODIFY TYPICAL DETAILS AS REQUIRED TO MATERIALS
- 1.3 THE CONTRACTOR SHALL EXAMINE THE DRAWINGS AND SHALL NOTIFY THE ENGINEER / ARCHITECT OF ANY DISCREPANCIES HE MAY FIND BEFORE PROCEEDING WITH THE WORK, OR DURING CONSTRUCTION.
- 1.4 IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE ADEQUATE SHORING AND BRACING FOR THE STRUCTURE FOR ALL LOADS THAT MAY BE IMPOSED DURING CONSTRUCTION.
- 1.5 ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE LATEST APPLICABLE STANDARD OR SPECIFICATIONS. ALL WORKS SHALL CONFORM WITH THE BEST PRACTICE PREVAILING IN THE VARIOUS TRADES.
- 1.6 ALL CONSTRUCTION AND WORKMANSHIP SHALL BE SUBJECT TO INSPECTION, EXAMINATION AND TESTING BY THE ENGINEER / ARCHITECT. THE ENGINEER / ARCHITECT SHALL HAVE THE RIGHT TO REJECT DEFECTIVE MATERIALS AND WORKMANSHIP OR REQUIRE ITS CORRECTION.
- 1.7 UNLESS SPECIFICALLY DETAILED ELSEWHERE, THE CONTRACTOR SHALL FOLLOW TYPICAL DETAILS AS SHOWN IN THESE DRAWINGS.
- 1.8 THE CONTRACTOR WILL BE RESPONSIBLE FOR THE COORDINATION OF WORK AMONG THE VARIOUS TRADES AS NECESSARY TO AVOID CONFLICTS AND TO
- ENSURE THE INSTALLATION OF ALL WORKS WITHIN THE AVAILABLE SPACE. 1.9 DO NOT SCALE DRAWINGS AND CALLED-OUT DIMENSIONS. STANDARD CODE REQUIREMENTS SHALL GOVERN OVER UNSCALED DRAWINGS.
- 1.10 SPECIAL NOTES AND DIMENSIONS INDICATED ON THE STRUCTURAL DRAWING SHALL BE COORDINATED WITH THE ARCHITECTURAL DRAWINGS. ARCHITECTURAL DRAWINGS SHALL BE USED TO DEFINE DETAIL CONFIGURATION, ELEVATIONS, OPENING JOINTS, SLOPES, ETC.
- 1.11 MODIFICATION OF SECTION AND SIZES OF STRUCTURAL MEMBERS SHALL NOT BE ALLOWED UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.
- 1.12 CONTRACTOR TO PROVIDE DYE PENETRANT/ULTRASONIC TESTING RESULT TO CLIENT. THESE TESTINGS SHALL BE CONDUCTED BY ACCREDITED AGENCY.
- 1.13 IN CASE STRUCTURAL MEMBERS SPECIFIED ARE NOT AVAILABLE, SUBMIT TO CLIENT ENGINEER AVAILABLE LIST OF MEMBERS FOR APPROVAL BEFORE PURCHASING.

DESIGN CRITERIA

1. LOADS

1.1 DEAD LOADS	
UNIT WEIGHT OF CONCRETE	24.00 KN/m ³ 1 8.00 KN/m
UNIT WEIGHT OF SOIL	1 8.00 KN/m ³
ROOFING (GI Sheet and Purlins)	0.3 7 k Pα
100mm CHB WALL	3. 17 k Pα
1.50mm CHB WALL	3.30 kPα
FLOOR FINISH	1 .53 k Pα
PARTITION LOAD	1 .00 k P a
CEILING	0.25 kPα
INSULATION	0.08 kPa
WATERPROOFING	0.26 kPa

1.2 LIVE LOADS ROOF OFFICE RESTROOM EXIT FACILITIES EVACUATION, BASIC FLOOR AREA	1.00 kPa 2.40 kPa 2.40 kPa 4.80 kPa 4.80 kPa
1.3 WIND LOAD	320 kph
1.4 SEISMIC LOADS SEISMIC ZONE FACTOR, Z NUMERICAL COEFFICIENT, RWX & RWZ IMPORTANCE FACTOR, I SITE COEFFICIENT, S (Sd) NQ NV FUNDAMENTAL PERIOD OF VIBRATION, T	0.40 8.50 1.50 4.00 1.128 1.456 C (hn) ^{3/4}

2. DESIGN CODE AND REFERENCE

HEIGHT IN METERS, h

ELECTRICAL/MECHANICAL/PLUMBING

THE FOLLOWING REFERENCES SHALL GOVERN THE DESIGN FABRICATION & CONSTRUCTION OF THE PROJECT:

AMERICAN CONCRETE INSTITUTE ACI 318 - 95 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

NATIONAL STRUCTURAL CODE OF THE PHILIPPINES (NSCP) VOLUME 1, PHILIPPINES 6th **EDITION, 2010**

ASSOCIATION OF STRUCTURAL ENGINEERS OF THE PHILIPPINES (ASEP) HANDBOOK OF STRUCTURAL STEEL SHAPES AND SECTIONS

FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) P-320/P-361

UBC 1997, STRUCTURAL ENGINEERING DESIGN PROVISION

STRUCTURAL DESIGN DATA AND SPECIFICATIONS A.B. CARILLO, 6th EDITION

1. CONCRETE

UNLESS INDICATED OTHERWISE ON PLANS, THE CONCRETE CLASS AND STRENGTH SHALL BE AS FOLLOWS:

STRUCTURAL ELEMENTS	CLASS	28-DAY CYLINDER STRENGTH MPa (psi)	MAX. SLUMP MM (in)
SLAB, STAIR, CURBS AND SLAB ON GRADE	"A"	20.7 (3000)	75 (3")
CAST-IN-PLACE GIRDERS, BEAMS, FOOTINGS AND COLUMN	"AA"	27.6 (4000)	100 (4")
OTHER STRUCTURAL ELEMENTS	"A"	20.7 (3000)	100 (4")
FOR NON STRUCTURAL MEMBERS		17.2 (2500)	100 (4")
LEAN CONCRETE	_	10.0 (1450)	75 (3")

ITEMS	AGGREGATE SIZE
FOOTINGS	25 MM (1")
SLABS, BEAMS, COLUMNS, OTHERS.	19 MM (3/4")
CURBS AND MASS, CONCRETE/SLAB ON GRADE	25 MM (1")

1.1 INFORM ARCHITECT/ENGINEERS OF OTHER MISCELLANEOUS CONCRETE STRUCTURAL ELEMENTS NOT SHOWN ABOVE TO DETERMINE THEIR RESPECTIVE COMPRESSIVE STRENGTH.

2. REINFORCING STEEL

- a. REINFORCING STEEL SHALL CONFORM TO LATEST EDITION OF ASTM A615 GRADE 60, DEFORMED, FOR 16MM DIA. BARS AND LARGER WITH MINIMUM YIELD STRENGTH fy = 414 MPa (60,000 PSI) AND ASTM A615 GRADE 40, DEFORMED, FOR 12MM DIA. BARS AND SMALLER WITH MINIMUM YIELD STRENGTH fy = $276MP\alpha$ (40,000 PSI).
- b. ALL REINFORCING BARS SHALL BE DEFORMED BARS UNLESS OTHERWISE SPECIFIED IN
- C. ALL REINFORCING BARS SHALL BE CLEAN OF RUST, GREASE OR OTHER MATERIALS LIKELY TO IMPAIR BOND.
- d. ALL REINFORCING BARS SHALL ACCURATELY AND SECURELY PLACED BEFORE POURING OF CONCRETE OR APPLYING MORTAR OR GROUT.

3. STRUCTURAL STEEL BOLTS/WELDS

MATERIAL	SPECIFICATIONS				
STEEL PLATES AND ROLLED SHAPES	DLLED SHAPES ASTM A36				
BOLTS	ASTM A325				
WELDS	AWS D1.1 - 183, E70XX SERIES				

CONSTRUCTION

1. SETTING OUT

 $0.35 \,\mathrm{kPa}$

0.0731

hn

ARCHITECT / ENGINEER

THE SETTING OUT AND THE ELEVATIONS OF THE DIFFERENT COMPONENTS OF THE STRUCTURE SHALL BE APPROVED BY THE ENGINEER PRIOR TO THE START OF ANY CONSTRUCTION WORK.

2. REINFORCED CONCRETE

a. CONCRETE MIX AND PLACING

- (1) DESIGN OF CONCRETE MIX SHALL MEET THE DESIGN CONCRETE STRENGTH GIVEN UNDER ITEM 1 OF MATERIALS.
- CONCRETE SHALL BE DEPOSITED, VIBRATED AND CURED IN ACCORDANCE WITH THE SPECIFICATIONS.
- FOR CONCRETE DEPOSITED AGAINST THE GROUND, LEAN CONCRETE WITH A MINIMUM THICKNESS OF 50mm SHALL BE LAID FIRST BEFORE INSTALLING THE REINFORCEMENT. THE LEAN CONCRETE SHALL NOT BE CONSIDERED IN MEASURING THE STRUCTURAL DEPTH OF CONCRETE SECTION.
- (4) THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR APPROVAL, THE POURING SEQUENCES FOR ALL CONCRETING WORK.
- (5) THE CONTRACTOR SHALL NOTIFY THE ENGINEER 48 HOURS PRIOR TO THE POURING OF ANY STRUCTURAL CONCRETE, SO AN INSPECTION CAN BE MADE ON ALL FORMS AND REINFORCING.
- (6) PREPARE AND SUBMIT CONCRETE MIX DESIGN INCLUDING AGGREGATES GRADATION, WATER AND CEMENT CONTENTS, AND CYLINDER STRENGTH TEST RESULT FOR REVIEW. CONCRETE MIX DESIGN SHALL BE TESTED AT 7,14 AND 28 DAY CURING PERIOD. THE TEST SHALL FOLLOW THE REQUIREMENT OF ASTM.
- USE OF ADMIXTURES IS PERMITTED TO PRODUCE PROPER SLUMP AND WORKABILITY BUT SUBJECT TO THE ENGINEER'S APPROVAL. ADDITION OF WATER TO CONCRETE AT JOB SITE IS NOT ALLOWED.

FOR CONCRETE SLAB, ALL REINFORCEMENT SHALL BE 0.02m CLEAR MINIMUM FROM TOP AND BOTTOM OF SLAB. TEMPERATURE BARS SHALL BE GENERALLY PLACED NEAR THE FACE IN TENSION AND SHALL NOT BE LESS THAN 0.0018 Bf.

FOR TWO OR MORE LAYERS OF REINFORCING BARS, USE SEPARATORS SPACED @ 0.90m O.C. AND IN NO CASE SHALL BE LESS THAN 2 SEPARATORS. CLEAR DISTANCE BETWEEN LAYERS SHOULD NOT BE LESS THAN 25mm OR BAR DIAMETER. FOR CAMBER:

COMPONENT	MINIMUM CAMBER
R.C. BEAMS	6mm FOR EVERY 4.50M. SPAN
CANTILEVER R.C. BEAM	18mm FOR EVERY 3.00M. SPAN
R.C. SLABS	3mm FOR EVERY 3.00M. SHORTER SPAN

- (10) COLUMN TIES SHALL BE PROTECTED BY A COVERING OF CONCRETE CAST MONOLITHICALLY WITH 0.05m THICK AND NOT LESS THAN 1/2 TIMES MAXIMUM SIZE OF COURSE AGGREGATES.
- (11) LOCATION OF ALL CONSTRUCTION OR COLD JOINTS MUST BE APPROVED BY THE ENGINEER/ARCHITECT.
- (12) PIPES OR DUCTS EXCEEDING ONE THIRD THE SLAB OR WALL THICKNESS SHALL NOT BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED. PIPES MAY PASS THROUGH STRUCTURAL CONCRETE IN SLEEVES BUT SHALL NOT BE EMBEDDED THEREIN.
- (13) REINFORCING BARS, ANCHOR BOLTS, AND OTHER INSERTS SHALL BE SECURED IN PLACE BEFORE POURING CONCRETE. BAR PLACEMENT AND SUPPORTS SHALL BE IN ACCORDANCE WITH THE RECOMMENDED ACI PRACTICE.
- (14) ALL INSERTS, ANCHOR BOLTS, ETC. TO BE EMBEDDED IN THE CONCRETE SHALL BE HOT DIP GALVANIZED UNLESS NOTED OTHERWISE.
- (15) IN GENERAL, THE LATEST EDITION OF THE MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES, ACI 315-99, SHALL BE ADHERED TO, UNLESS SHOWN OTHERWISE.

b. BAR BENDING, SPLICING AND PLACING

- (1) THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR APPROVAL ALL SHOP DRAWINGS INDICATING THE BENDING, CUTTING, SPLICING AND INSTALLATION OF ALL REINFORCING BARS.
- BARS SHALL BE BENT COLD, BARS PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT UNLESS PERMITTED BY THE ENGINEER.
- (3) BAR SPLICING NOT INDICATED ON DRAWINGS SHALL BE SUBJECT TO THE APPROVAL
- WELDED SPLICES, IF APPROVED BY THE ENGINEER, SHALL DEVELOP IN TENSION AT LEAST 125% OF THE SPECIFIED YIELD STRENGTH OF THE BARS.
- (5) LAPPED SPLICES SHALL BE STAGGERED WHERE POSSIBLE.
- (6) IN GENERAL, BAR SPLICES SHALL BE MADE AT POINTS OF MINIMUM STRESS. SPLICES SHALL BE SECURELY WIRED TOGETHER STAGGER SPLICES AT LEAST 600MM. WHENEVER POSSIBLE IN BEAMS AND SLABS. SPLICE TOP BARS AT MID SPAN AND BOTTOM BARS NEAR SUPPORT, SPLICE OF REINFORCEMENT SHALL BE MADE ONLY AS REQUIRED OR PERMITTED ON DESIGN DRAWINGS OR AS ALLOWED BY THE ACI CODE OR AS AUTHORIZED BY THE ENGINEERS.
- (7) BARS NOTED AS CONTINUOUS SHALL HAVE A MINIMUM SPLICE LENGTH OF 42 BAR DIAMETER BUT NOT LESS THAN 60MM. UNLESS OTHERWISE NOTED.
- REINFORCEMENTS SHALL BE SPLICED ONLY AS INDICATED ON THE DRAWINGS.
- ANY WELDING TO BE PERFORMED MUST HAVE PRIOR WRITTEN APPROVAL OF THE ENGINEER.
- (10) WELDING OF REINFORCING STEEL IS NOT PERMITTED UNLESS OTHERWISE SHOWN ON THE DRAWING. WELDING OF REINFORCING STEEL SHALL CONFORM TO AWS D1.4-79 *AWS STRUCTURAL WELDING CODE* OF THE AMERICAN WELDING SOCIETY. REINFORCING STEEL WHICH IS WELDED SHALL CONFORM TO ASTM A 706. REINFORCING STEEL NOT CONFORMING TO ASTM A 706 MAY BE USED IF MATERIAL PROPERTIES OF THE REINFORCING STEEL CONFORM TO AWS D1.4-79.
- (11) ANCHOR BOLTS, DOWELS AND OTHER EMBEDDED ITEMS ARE TO BE SECURELY TIED IN PLACE BEFORE CONCRETE IS POURED.

(12) TYPICAL HOOPS & SUPPLEMENTARY TIE DETAIL

HOOK AT EACH END FOR 10ø, 12ø, 16ø

COLUMN HOOPS

CONSECUTIVE CROSSTIES

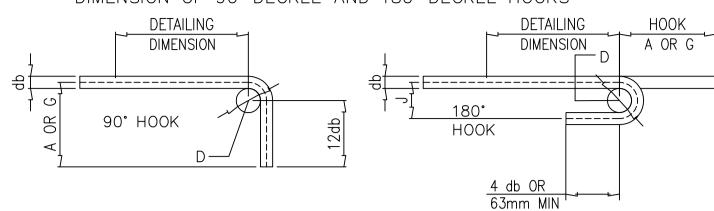
ALTERNATIVE 90° & 135° ENDS ON

SUPPLEMENTARY DETAIL HOOPS MAIN DETAIL CROSSTIE OVERLAPPING HOOPS

APPROVED FOR IMPLEMENTATION BY OWNER:

(13) TYPICAL STANDARD HOOK DETAIL

DIMENSION OF 90-DEGREE AND 180-DEGREE HOOKS



PIN DIAMETER: D = 6db FOR DIAMETER 10 THRU DIAMETER 25 D = 8db FOR DIAMETER 28, DIAMETER 32 AND DIAMETER 36

RECOMMENDED END HOOKS, ALL GRADES

			180 DEGR	EE HOOK	90 DEGREE HOOK		
		FINISHED BEND		LL HOOK			
	BAR SIZE	DIAMETER, D IN. (mm.)	A OR G	J	A OR G		
		DIAMETER, D III. (IIIII.)	(mm.)	(mm.)	(mm.)		
	10mmø	60	130	80	155		
	12mmø	80	155	105	205		
	16mmø	100	180	130	255		
	20mmø	115	205	155	305		
	25mmø	155	280	205	410		
	28mmø	245	285	300	485		
	32mmø	275	435	340	560		
	36mmø	305	485	375	610		

(14) TYPICAL STANDARD STIRRUPS AND TIE HOOK DETAIL

GENERAL USE SEISMIC LOAD 12db FOR 19, 22, & 25 6db FOR 10, 12, & 16 GENERAL USE GENERAL USE -CENTER LINE OF -CENTER LINE OF BEAM -CENTER LINE OF BEAM BEAM

STIRRUPS AND TIE HOOKS, ALL GRADES							
BAR	, D ,	GE	ENERAL U	SEISMIC USE			
SIZE	(mm)	90. HOOK	135°	HOOK	135°	, HOOK	
		A OR G	A OR G H APPROX.		A OR G	H APPROX.	
10mmø	40	105mm	105mm	55mm	130mm	90mm	
12mmø	55	115mm	115mm	15mm 80mm		145mm	
16mmø	65	155mm	140mm	100mm	205mm	140mm	
20mmø	145	305mm	200mm	145mm	275mm	170mm	
25mmø	155	410mm	265mm 145mm		365mm	230mm	

(15) IF BEAM REINFORCING BARS END IN A WALL, THE CLEAR DISTANCE FROM THE BAR TO THE FARTHER FACE OF THE WALL SHALL NOT BE LESS THAN 50mm MINIMUM EMBEDMENT LENGTH SHALL BE AS SHOWN AS IN TABLE "A".

	TABLE A								
BAR	DEVELOP	DEVELOPMENT LENGTH, Ld, IN TENSION							
SIZE	F'c = 3,000	psi(20.68 Mpa.)	F'c=4,000 p	si(27.58 Mpa.					
	Top Bars	Other Bars	Top Bars	Other Bars					
10mmø	525	405	455	350					
12mmø	700	540	605	465					
16mmø	870	670	755	580					
20mmø	1045	805	905	700					
25mmø	1395	1075	1205	930					
28mmø	1570	1210	1360	1050					
32mmø	1770	1360	1535	1180					

	TABLE B									
)	BAR SIZE	MINIMUM LENGTH OF COMPRESSION LAP SPLICE (MM)								
	10mmø	305								
	12mmø	385								
	16mmø	480								
	20mmø	575								
	25mmø	765								
	28mmø	860								
	32mmø	970								

TADLED

GENERAL NOTES SCALE 1:125



RNFA STRUCTURAL ENGINEER PTR No.: Place of Issue : ---Date of Issue : ---

MULTI-PURPOSE CENTER CITY OF BORONGAN, EASTERN SAMAR

PROJECT / LOCATION:



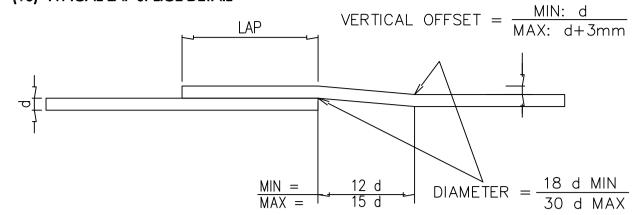
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MARCO BOASSO CHIEF OF MISSION IOM PHILIPPINES DIRECTOR MANILA GLOBAL ADMINISTRATIVE CENTER

NO.	REVISIONS	DATE	BY	SHEET CONTENT					SHEET NO.
1	ISSUED FOR BIDDING	23SEP15	MVA						
2	ISSUED FOR CONSTRUCTION	200CT15	MVA	GENERAL NOTES			CTN1		
W	REVISION - A	19 JUL 16	MVA		GLIV	LIVAL NOTE	.5	L	2101
				CHECKED AQT	DRAWN	MAApelo	FILENAME	PRO.	
·		·	·	APPROVED RNF	DATE	19 JUL 16	2K1404A-ST01		2K1404A

GENERAL NOTES

(16) TYPICAL LAP SPLICE DETAIL



C. CONCRETE COVER TO REINFORCEMENT

CLEAR CONCRETE COVERING OVER REINFORCING BARS SHALL BE AS FOLLOWS:

COLUMN TO TIES **BEAM TO STIRRUPS** FOOTING

40mm 40mm 75mm

d. CONSTRUCTION JOINT

- (1) THE POSITION AND FORM OF ANY CONSTRUCTION JOINT SHALL BE AS SHOWN ON DRAWINGS OR AS AGREED WITH THE ENGINEER.
- (2) ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE ROUGHENED TO 6MM. AMPLITUDE.
- (3) ALL CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE DUST, CHIPS, OR OTHER FOREIGN MATTERS PRIOR TO PLACING OF ADJACENT CONCRETE.

e. FALSEWORK

ALL FALSEWORK SHALL BE DESIGNED BY THE CONTRACTOR SUBJECT TO THE APPROVAL BY THE ENGINEER.

- (1) FORMS SHALL BE PROVIDED FOR ALL CONCRETE INDICATED UNLESS SPECIFIED OTHERWISE. FORMS SHALL BE SET TRUE TO LINE AND GRADE AND MAINTAINED SO AS TO ENSURE COMPETENT WORK WITHIN THE ALLOWABLE TOLERANCES SPECIFIED AND SHALL BE MORTAR TIGHT.
- (2) FORMS AND THEIR SUPPORTS SHALL BE DESIGNED SO AS NOT TO DAMAGE PREVIOUSLY PLACED STRUCTURE.
- (3) NO CONSTRUCTION LOAD SHALL BE SUPPORTED ON, NOR ANY SHORING REMOVED FROM ANY PART OF STRUCTURE UNDER CONSTRUCTION EXCEPT WHEN THAT PORTION OF THE STRUCTURE IN COMBINATION WITH THE REMAINING FORMING AND SHORING SYSTEM HAS SUFFICIENT STRENGTH TO SUPPORT SAFELY ITS WEIGHT AND ADDITIONAL IMPOSED LOAD.
- FORMS SHALL BE REMOVED IN SUCH MANNER AS NOT TO IMPAIR SAFETY AND SERVICEABILITY OF THE STRUCTURE.
- (5) SHORING (TUKOD) FOR BEAMS/SLABS SHOULD BE REMOVED AFTER 14th DAY
- SCHEDULE OF STRIPPING OF FORMS AND SHORES.

REMOVAL OF FORMS & SHORING						
STRUCTURAL COMPONENT	MIN.TIME PERIOD (DAYS)					
FOUNDATION	_	1				
WALL, COLUMN, BEAMS GIRDER SIDES & SLAB ON GRADE	_	3				
	UNDER 3.00 M.	7				
JOIST, BEAMS & GIRDER SOFFIT	3.00 M. to 6.00 M.	14				
	OVER 6.00 M.	21				
	UNDER 3.00 M.	4				
ONE-WAY FLOOR SLABS	3.00 M. to 6.00 M.	7				
	OVER 6.00 M.	10				

g. PROTECTION AND CURING OF CONCRETE

CONCRETE SURFACES SHALL BE PROTECTED FROM HARMFUL EFFECTS OF SUN, WIND AND RUNNING WATER AND SHALL BE KEPT DAMP FOR AT LEAST 7 DAYS.

3. STRUCTURAL STEEL

- (1) ALL STRUCTURAL MILL SECTIONS, AND BUILT-UP PLATE SECTIONS SHALL BE CONSTRUCTED IN ACCORDANCE WITH AISC LATEST "SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS"
- STEEL PLATES, SHAPES, BARS, AND METAL FABRICATIONS ARE ASTM A-36 UNLESS NOTED OTHERWISE.
- (3) SCHEDULE OF BOLTS/NUTS/WASHERS

BOLTS	TYPE/GRADE	NUTS
ASTM A307	GRADE A	ASTM A563
ASTM 325 (HIGH-STRENGTH BOLTS)	TYPE 2	ASTM A563
ASTM 490 (QUENCHED AND TEMPERED ALLOY)	TYPE 2	ASTM A563

BOLTS	WASHERS	REMARKS
ASTM A307	ASTM F436	
ASTM 325 (HIGH-STRENGTH BOLTS)	ASTM F436	
ASTM 490 (QUENCHED AND TEMPERED ALLOY)	ASTM F436	ø25MM AND ABOVE

(4) HOLE SIZE FOR ANCHOR BOLTS

BOLT SIZE Ø (MM.)	HOLE SIZE Ø (MM.)
19 TO 25 INCL	DIAM + 8
OVER 25 TO 50 INCL	DIAM + 12
OVER 50	DIAM + 25
SWEDGE TYPE	DIAM + 19

NOTES: A490 BOLTS SHOULD NOT BE HOT-DIPPED GALVANIZED

- (5) UNFINISHED BOLTS SHALL CONFORM TO ASTM A-307 GRADE A. HIGH STRENGTH BOLT SHALL CONFORM TO ASTM A325 OR ASTM A490 AS NOTED. USE 16 MM DIAMETER A325 BOLTS FOR ALL BEAM TO BEAM, BEAM TO GIRDER/COLUMN, GIRDER TO COLUMN BOLTED CONNECTION. USE TWO BOLTS MIN. UNLESS NOTED OTHERWISE.0
- (6) ALL HIGH STRENGTH BOLTS A325 OR A490 SHALL BE SLIP CRITICAL (A325-SC OR A490-SC CLASS A) UNLESS NOTED OTHERWISE. THE INSTALLATION OF HIGH STRENGTH BOLTS SHALL CONFORM TO THE LATEST EDITION OF AISC SPECIFICATION FOR STRUCTURAL JOINT USING ASTM A325 OR A490 BOLTS. WHERE NON SLIP CRITICAL BOLTS ARE SPECIFIED. THESE BOLTS SHALL ONLY BE TIGHTENED TO A SNUG TIGHT CONDITION.
- BOLT HOLE IN STEEL SHALL BE 1.6MM LARGER IN DIAMETER THAN THE DIAMETER OF BOLT USED FOR SLIP CRITICAL CONNECTIONS CONSTRUCTION" OR SHORT SLOTTED HOLES FOR NON-SLIP CRITICAL CONNECTION AS NOTED, UNLESS OTHERWISE SPECIFIED.
- ELECTRODES FOR WELDING: ASTM 233 E-70XX SERIES; COMPLY WITH AWS D1.1 CODE REQUIREMENTS.
- (9) FLAME CUTTING AND WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST "STANDARD CODE FOR WELDING IN BUILDING OF THE AMERICAN WELDING SOCIETY.
- (10) ALL BUTT WELDS SHALL BE FULL PENETRATION AND SHALL BE PROPERLY BACK-CHIPPED OR GOUGED. BACK-UP PLATES SHALL BE PROVIDED AS REQUIRED.
- (11) GRIND ALL EXPOSED WELDS SMOOTH, EXCEPT FILLET WELDS.
- (12) WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. FILLET WELD SIZES ARE THE WIDTH OF THE HORIZONTAL OR VERTICAL LEG. WHERE LENGTH OF WELD IS NOT SHOWN IT SHALL BE FULL LENGTH OF JOINT. WELDING ELECTRODES TO BE E70XX UNLESS NOTED OTHERWISE.
- (13) ALL LEVEL WELDS ARE FULL PENETRATION, UNLESS NOTED OTHERWISE. SIZE ALL FILLET WELDS PER AWS WHERE NOT SHOWN WITH WELD SIZE, PROVIDE MIN. WELD SIZE TO DEVELOP TENSION OR SHEAR CAPACITY OF THE SMALLER MEMBER OF THE PIECES BEING CONNECTED (4.76MM MIN.)
- (14) THE CONTRACTOR SHALL PROVIDE MINIMUM 10MM. CONCRETE COVER AROUND ALL STEEL MEMBERS / COMPONENTS (WF, TS, PLATES, BOLTS, ETC.) ADJACENT TO
- (15) WELDED CONNECTIONS BETWEEN MEMBERS OF MOMENT FRAMES SHALL BE TESTED BY NON- DESTRUCTIVE METHOD.
- (16) APPLY TT-P-645 SHOP PAINT FOR ALL FABRICATIONS.
- (17) SHOP PAINTING FOR STRUCTURAL STEEL SHALL BE RUST INHIBITIVE PRIMER WITH MINIMUM D.F.T. 2.0 MILS.
- (18) TOUCH-UP PAINTING: APPLY PAINT TO EXPOSED AREAS IN MANNER SATISFACTORY TO THE ENGINEER WITH SAME MATERIAL AS SHOP PAINT.
- (19) COMPLY WITH AISC CODE AND SPECIFICATION FOR BEARING, ADEQUACY OF TEMPORARY CONNECTIONS AND ALIGNMENT.
- (20) CONTRACTOR SHALL FURNISH COMPLETE ERECTION DRAWINGS FOR THE PROPER IDENTIFICATION AND ASSEMBLY OF ALL BUILDING COMPONENTS. THESE DRAWINGS WILL SHOW ANCHOR BOLTS SETTING. PRIMARY SECONDARY, AND ROOF FRAMING, AND NECESSARY INSTALLATION DETAILS. SUBMIT SHOP DRAWINGS FOR APPROVAL BEFORE FABRICATION.
- (21) THE STEEL SUBCONTRACTOR SHALL COMPLY WITH THE LATEST AISC CODE OF STANDARD PRACTICE.

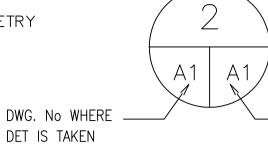
PROJECT / LOCATION :

- THE STEEL SUBCONTRACTOR SHALL DETERMINE THE ERECTION SEQUENCE FOR ALL STEEL WORKS. THE STEEL SUBCONTRACTORS SHALL ALSO COORDINATE WITH OTHER TRADES AND SITE CONDITIONS IN DETERMINING THE PROPER STEEL ERECTION SEQUENCE SO AS NOT TO DAMAGE WORK PERFORMED BY OTHER TRADES AND / OR PREVIOUSLY ERECTED STEEL MEMBERS.
- (23) WORK POINTS, MEMBERS LENGTH AND/OR ERECTION SEQUENCE SHALL BE ADJUSTED BY THE STEEL SUBCONTRACTOR TO MINIMIZE THE EFFECT OF THE TEMPERATURE CHANGES AND DIFFERENTIAL TEMPERATURE EFFECTS. TEMPERATURE EFFECTS (SUCH AS EXPOSED TO STRONG SUN ON ONE SIDE OF THE BUILDING). MEETING AISC ACCEPTABLE MILL STANDARD AND ERECTION TOLERANCES.
- (24) ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36, Fy=248 MPa (36,000 PSI)
- (25) FABRICATOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL BY THE ENGINEER AND THE OWNER PRIOR TO FABRICATION.

4. FOOTINGS

- (1) THE ASSUMED SOIL BEARING CAPACITY SHALL BE 100 KPQ AT 1.5M FROM NATURAL GRADE LINE TO BOTTOM OF FOOTING.
- (2) FOOTING SHALL REST ON 100mm THK GRAVEL BASE COURSE COMPACTED TO 95% MAXIMUM DENSITY.
- BACKFILL SHALL BE PLACED IN 150mm LAYERS AND EACH LAYER SHALL BE COMPACTED TO A MINIMUM OF 95% MAXIMUM DENSITY, SHALL BE FREE FROM DETRIMENTAL AMOUNTS OF ORGANIC MATERIAL & NO ROCK OR SIMILAR IRREDUCIBLE MATERIAL W/ A MAXIMUM DIMENSION GREATER THAN 300mm BE BURIED OR PLACED IN FILLS.
- (4) ALL EXCAVATIONS, BACKFILLING AND COMPACTION SHALL BE INSPECTED AND APPROVED BY THE CONTRACTING OFFICER.
- (5) THE CONTRACTOR SHALL VERIFY THE ACTUAL SOIL CONDITION BEFORE CONSTRUCTION OR AFTER FOOTING EXCAVATION IS DONE, TO CHECK THE GEOTECHNICAL REPORTS RECOMMENDED BEARING CAPACITY, IF ANY.
- (6) NO FOOTING SHALL REST ON FILL.
- MINIMUM CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE 75mm FOR CONCRETE DEPOSITED AGAINST THE GROUND.
- CONTRACTOR TO PROVIDE FOR DE-WATERING OF EXCAVATIONS FROM EITHER SURFACE WATER, GROUND WATER OR SEEPAGE.
- CONTRACTOR SHALL PROVIDE FOR DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING AND SHORING REQUIRED TO SAFELY RETAIN THE EARTH BANKS.
- (10) ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE WALLS HAVE ATTAINED FULL DESIGN STRENGTH. CONTRACTOR SHALL BRACE OR PROTECT ALL BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED FULL STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS AND INSTALLATION OF SUCH BRACING.
- (11) FOOTINGS SHALL BE PLACED AND ESTIMATED ACCORDING TO DEPTHS SHOWN ON DRAWINGS. SHOULD SOIL ENCOUNTERED AT THESE DEPTHS NOT BE APPROVED BY THE FOUNDATION ENGINEER, FOOTING ELEVATIONS WILL BE ALTERED AS REQUIRED.
- (12) FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN BUILDING AREA SHALL BE MECHANICALLY COMPACTED IN LAYERS, IN ACCORDANCE WITH SOIL REPORT. FLOODING WILL NOT BE PERMITTED.

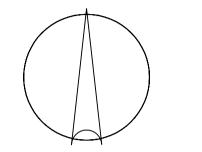




MAIN TITLE TARGET

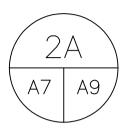
IDENTIFICATION SYMBOL

DWG. No WHERE DET IS SHOWN

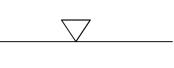


NORTH ARROW

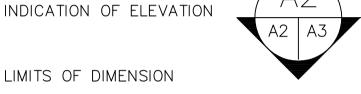
OR SIMILARITY



SUB-TITLE TARGET



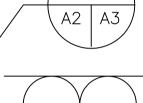




SECTION TARGET



SECTION IN EARTH



BUNDLED BARS

DETAIL REFERENCE TARGET

SHEET NO.

2K1404A



SECTION IN CONCRETE



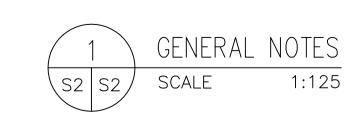
ON CENTER

DIAMETER **SQUARE**

AND CENTERLINE

ABBREVIATIONS

COL COLUMN CONC CONCRETE CONST CONSTRUCTION CONT CONTINUOUS CTR CENTER DET DETAIL DIA DIAMETER DWG DRAWING EA EACH EF EACH FACE ENGR ENGINEER EQ EQUAL EW EACH WAY EXP EXPANSION EXT EXTERIOR EXT EXTERIOR EXT EXTERIOR EXT EXTERIOR EXT EXISTING FF FAR FACE FFL FINISH FLOOR LINE FIN FINISH FTG FOOTING GEN GENERAL GRND GROUND HOR HORIZONTAL TYP INT INTERIOR KAPA MAX MAX MAX MAX MAX MAX MAX	METER MILLIMETER MAXIMUM MINIMUM MEGAPASCAL NEWTON NEAR FACE NUMBER ON CENTER QUANTITY RADIUS REINFORCED CONCRETE REINFORCEMENT SIDEWALK SLOPE SPIRAL SPACES STANDARD STIRRUP SYMMETRY TOP BARS THICK TYPICAL VARIABLE		
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RNFA STRUCTURAL ENGINEER PTR No. : Place of Issue : ---Date of Issue : ---

ARCHITECT / ENGINEER

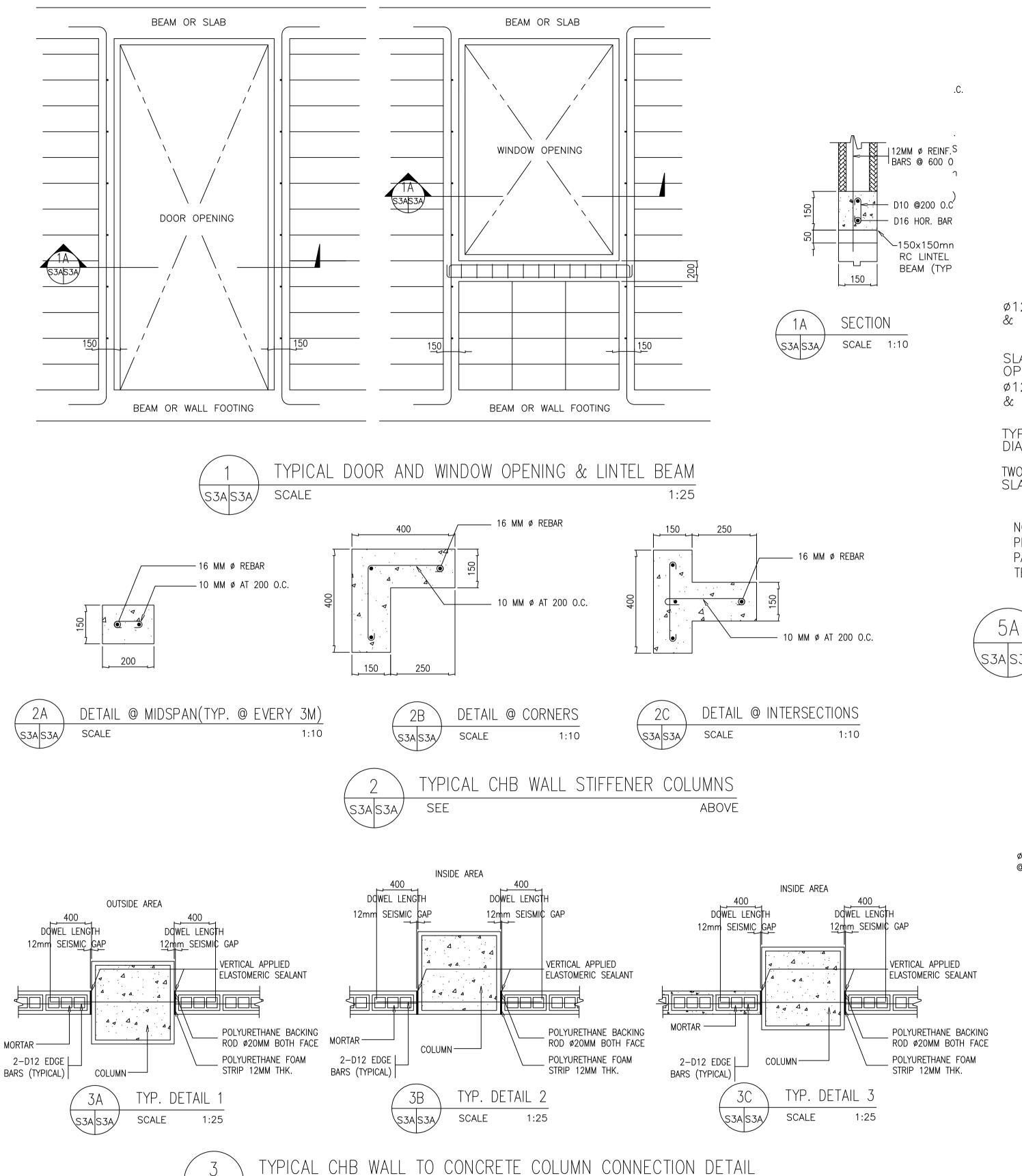
MULTI-PURPOSE CENTER CITY OF BORONGAN, EASTERN SAMAR



IOM/UNICEF



	NO.	REVISIONS	DATE	BY	NO.	REVISIONS	DATE	BY	SHEET CONTEN	Γ			
		ISSUED FOR BIDDING	23SEP15	MVA						GENEF	RAL NOTE		Ε
ı									CHECKED AQ	T DRAWN	MAApelo		PRO
nicet									APPROVED RNI	DATE	23SEP15	2K1404A-ST02	



Avm=600MM O.C. __Ahm __GROUTED CELL

CCHEDITE OF CONCRETE HOLLOW BLOCKS

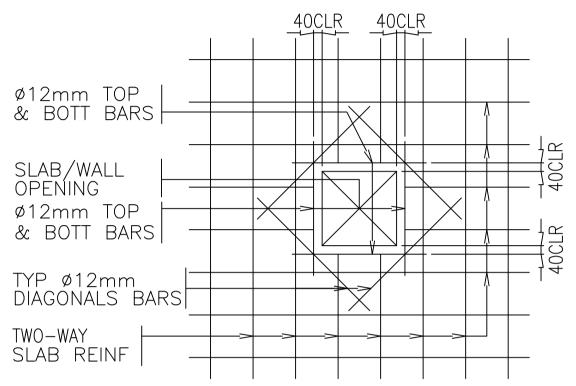
CHEDULE								
t (MM.)		RTICAL ORCEMEN	Т	HORIZONTAL REINFORCEMENT				GROUTED CELL
()	Ø	Avm		Ø		Ahm		
150	12	600 MM	O.C.	10	600	ММ	o.c.	ALL
100	12	600 MM	O.C.	10	600	ММ	O.C.	ALL

Avm = VERTICAL REINFORCEMENT SPACING Ahm = HORIZONTAL REINFORCEMENT SPACING t = THICKNESS OF MASONRY

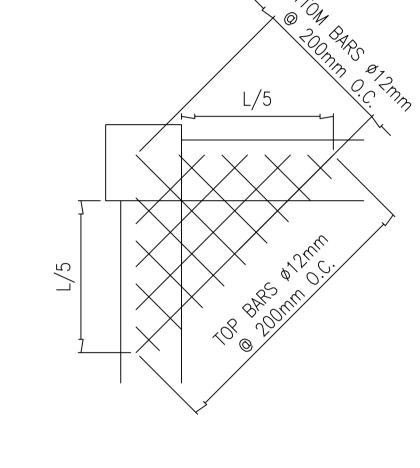
- 1. ALL EXTERIOR CONCRETE BLOCKS SHALL HAVE A MINIMUM STRENGTH OF 350 PSI (150MM THK)
- 2. ALL INTERIOR CONCRETE BLOCKS SHALL HAVE A MINIMUM STRENGTH OF 350 PSI (100MM THK) 3. PROVIDE STIFFENER COLUMNS AT EVERY THREE METERS (3.00 M.) O.C. WITH 2-16Ø VERTICAL BAR
- WITH 10Ø TIES @ 200MM O.C. AS INDICATED.

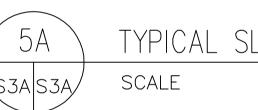
 4. PROVIDE STIFFENER BEAMS (200MM X150 MM) AT EVERY THREE METERS (3.00 M.) O.C. WITH 2-16Ø
- HORIZONTAL BARS WITH 100 TIES AT 200MM O.C.
- 5. ALL CHB CELLS CONTAINING REINFORCEMENT SHALL ONLY BE SOLIDLY FILLED WITH GROUT, ALSO ALL CHB CELLS BELOW FINISH GRADE LINE SHALL BE SOLIDLY FILLED WITH GROUT.
- 6. UNLESS OTHERWISE NOTED, SOLID GROUT SHALL MEAN ALL CELLS (WITH OR WITHOUT
- REINFORCEMENT BE GROUTED.)
- 7. SPECIAL FIELD INSPECTION IS REQUIRED.
- 8. LAYING OF CONC. BLOCKS SHOULD BE 1200MM HIGH IN ONE DAY (1 DAY).





PROVIDE THESE ADDITIONAL BARS FOR ALL OPENINGS PLUS BARS PARALLEL TO SIDE OF OPENING EQUAL TO THE NUMBER OF TERMINATED BARS BY THE OPENING.





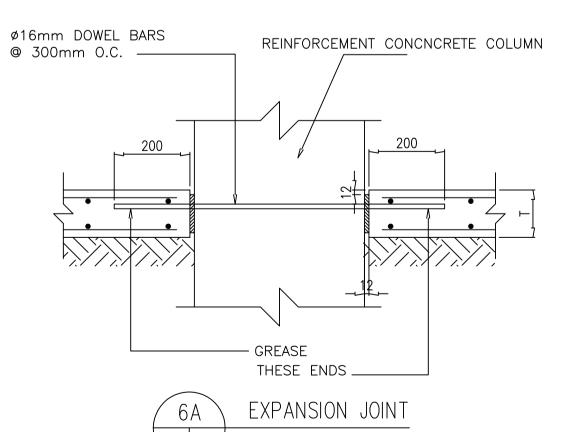
typical slab/wall opening detail 1:25 S3A|S3A

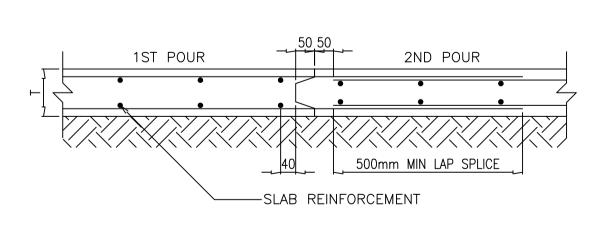
TYPICAL EXTERIOR CORNER DETAIL 1:35

SCALE

- 1. ALL REINFORCEMENT SHALL BE 0.019M CLEAR MINIMUM FROM TOP AND BOTTOM SLAB.
- 2. FOR TWO-WAY SLAB ALONG THE LONGER SPAN AND SHALL BE PLACED BELOW THOSE ALONG THE LONGER SPAN AT CENTER AND OVER THE LONGER SPAN BARS NEAR THE SUPPORT UNLESS OTHERWISE INDICATED OR SHOWN IN DETAIL. THE SPACING OF THE BARS AT THE COLUMN STRIPS
- CAN BE APPROXIMATELY 11/3 OF THE MIDDLE STRIP IN NO CASE GREATER THAN 21/3 THE SLAB THICKNESS.

 3. UNLESS OTHERWISE DETAILED IN CONDITIONING SLAB HAVING SAME REINFORCEMENT RUNNING IN ONE DIRECTION, REINFORCING SHALL BE
- BENT, EXTENDED OR CUT AS FOLLOWS.
- 4. TEMPERATURE BARS FOR SLAB SHALL BE GENERALLY PLACED NEAR THE TENSION FACE AND SHALL NOT BE LESS THAN 0.025 BT.
- 5. UNLESS OTHERWISE NOTED, ALL BEDDED SLAB SHALL BE REINFORCED WITH 10mm@ BAR @0.25M.O.C. B.W. AT CENTER OF SLAB. CONSTRUCTION JOINT FOR SLAB SHALL NOT BE MORE THAN 3.0M APART.
- 6. PROVIDE EXTRA REINFORCEMENT AT CORNER SLAB (TWO ADJACEN DISCONTINUOUS EDGE) AS SHOWN ABOVE.
- 7. PROVIDE SUPPLEMENTARY SMALL UNFRAMED OPENING SLAB AS SHOWN BELOW.





1:10

CONSTRUCTION JOINT 1:10 S3AS3A SCALE





RNFA STRUCTURAL ENGINEER Place of Issue : ---PTR No.: Date of Issue : ---

SEE

ARCHITECT / ENGINEER :

\S3A|S3A,

MULTI-PURPOSE CENTER CITY OF BORONGAN, EASTERN SAMAR IOM • OIM

OWNER:

ABOVE

PROJECT / LOCATION:



MARCO BOASSO CHIEF OF MISSION IOM PHILIPPINES DIRECTOR MANILA GLOBAL ADMINISTRATIVE CENTER

SCALE

S3A|S3A

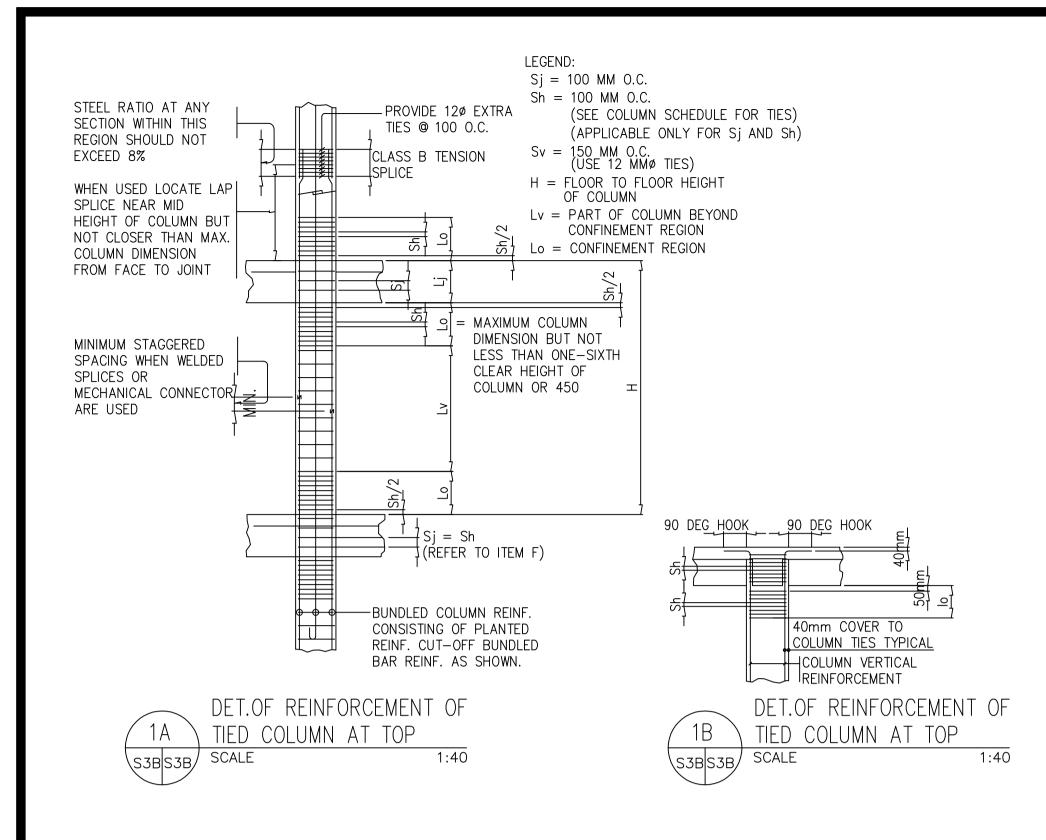
APPROVED FOR IMPLEMENTATION BY OWNER:

	1	ISSU
	2	ISSU
	3	REV
₹		

REVISIONS DATE BY SHEET CONTENT UED FOR BIDDING 23SEP15 MVA TYPICAL DOOR AND WINDOW OPENING & LINTEL BEAM
TYPICAL CHB WALL STIFFENER COLUMNS
TYPICAL CHB WALL TO CONCRETE COLUMN CONN. DETAIL
TYPICAL CONCRETE HOLLOW BLOCK PLAN
CONSTRUCTION JOINTS FOR SLAB ON FILL UED FOR CONSTRUCTION 200CT15 MVA 19JUL16 MVA /ISION - A

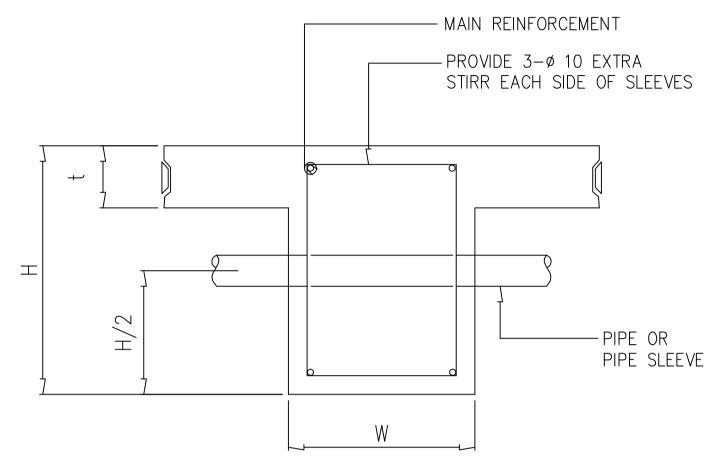
AQT DRAWN MAApelo FILENAME 2K1404A-ST3A 2K1404A APPROVED RNF DATE 19JUL16

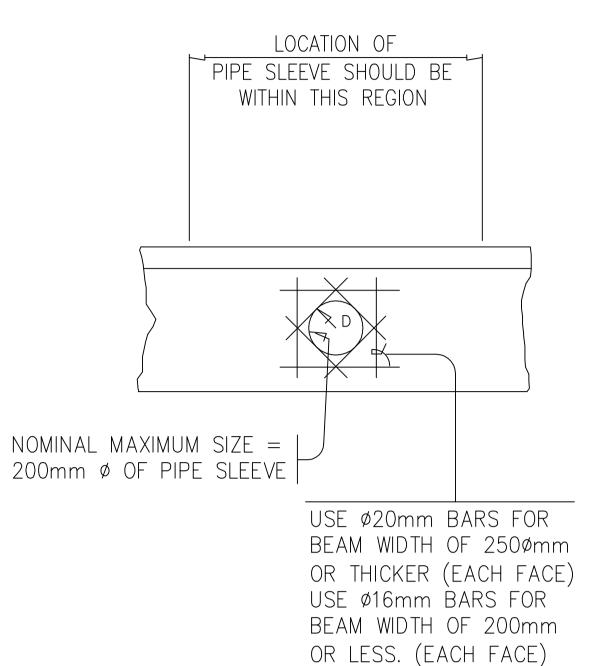
SHEET NO.



NOTE:

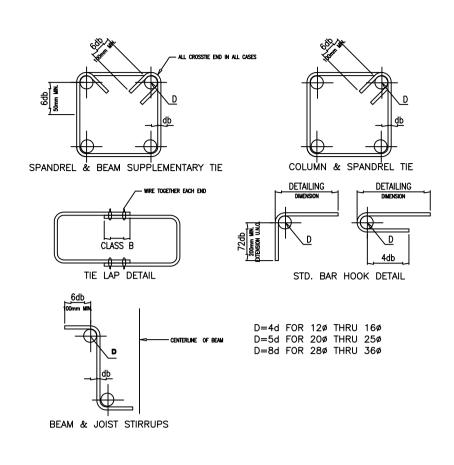
1. PIPES OR PIPE SLEEVES SHALL NOT BE LARGER IN DIAMETER THAN 1/3 THE OVERALL DEPTH OF BEAM OR GIRDER. 2. THEY SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTER. 3. THEY SHALL NOT IMPAIR SIGNIFICANTLY THE STRENGTH OF CONSTRUCTION. 4. SUBMIT TO STRUCTURAL ENGINEER FOR APPROVAL THE LOCATION OF EMBEDDED PIPE OR PIPE SLEEVE PRIOR TO INSTALLATION.





TYPICAL PIPE SLEEVES DETAIL

1:25



SCALE

TYPICAL BAR BENDING DETAIL

NOTES ON COLUMNS:

- A. FOR ALL TIED COLUMNS, MIN. 4 LAYERS OF TIES SHALL BE PROVIDED AT 100mm O.C. IN COLUMN REINFORCEMENT JUST ABOVE THE GIRDER IN BOTH SIDES OF COLUMN CONST. JOINTS.
- B. COLUMN TIES & SPIRALS SHALL BE PROVIDED WITH EVERYWHERE BY COVERING OF CONCRETE CAST MONILITHICALLY WITH THE CORE OF MIN. THICKNESS OF 3.81 CM. (1-1/2") MAX. SIZE OF COARSE AGGREGATE.
- C. WHERE COLUMNS CHANGE IN SIZE, VERT. REINF'T. SHALL BE OFFSET AT SLOPE OF NOT MORE THAN 1:6 & EXTRA 10mm dia. TIES AT 100mm SHALL BE PROVIDED THROUG—OUT THE OFFSET REGION.

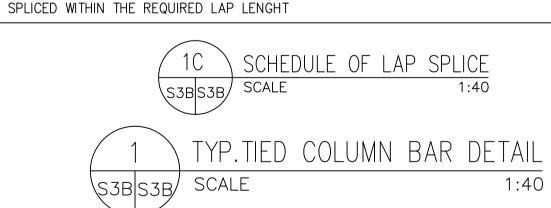
SCHEDULE OF LAP SPLICE AND ANCHORAGE LENGTH									
BAR SIZES (MM)	LAP SPLICE	CES LENGTH ANCHORA							
(IVIIVI)	TYPE " A "	LENGTH (MM)							
ø 10	400	300	600						
ø 12	450	350	600						
ø 16	610	400	600						
ø 20	760	560	600						
ø 25	1100	900	680						
ø 28	1400	1200	560						
ø 32	1900	1400	1120						
ø 36	2410	2410	1430						
NOTES:									

NOTES:

1. TYPE "A"BUNDLED BARS

TYPE "B" INDIVIDUAL BARS

2. NOT MORE THAN 33% OF THE BARS SHALL BE



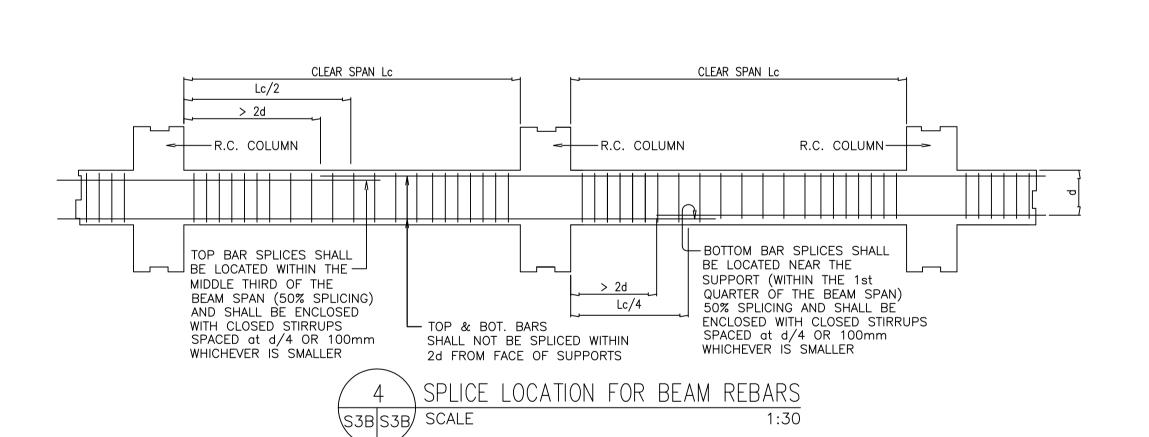


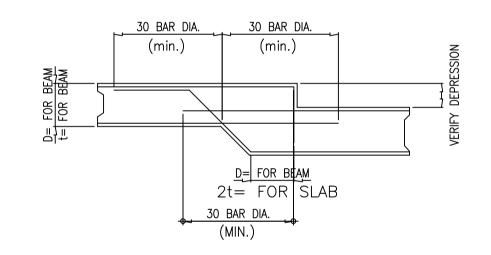


1:25

PIPE SLEEVES DETAIL

S3BS3B SCALE

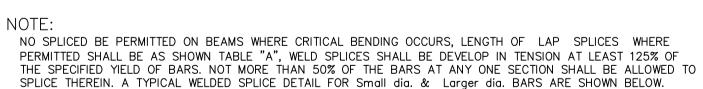


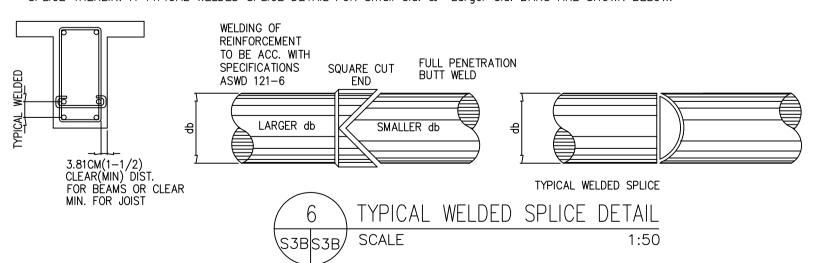


5 TYP. DROP SLAB DETAILED
S3BS3B SCALE 1:75

SHEET NO.

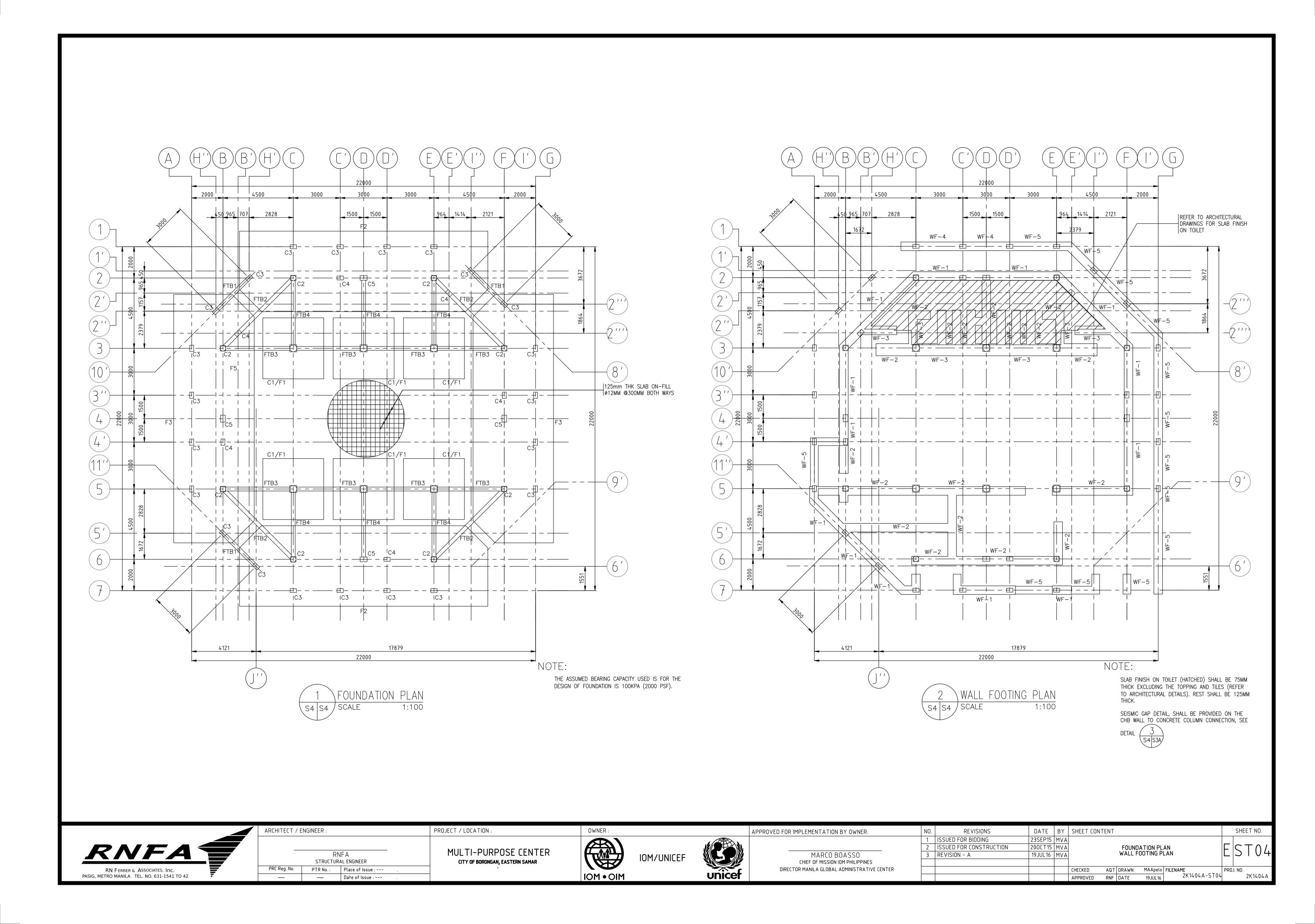
2K1404A

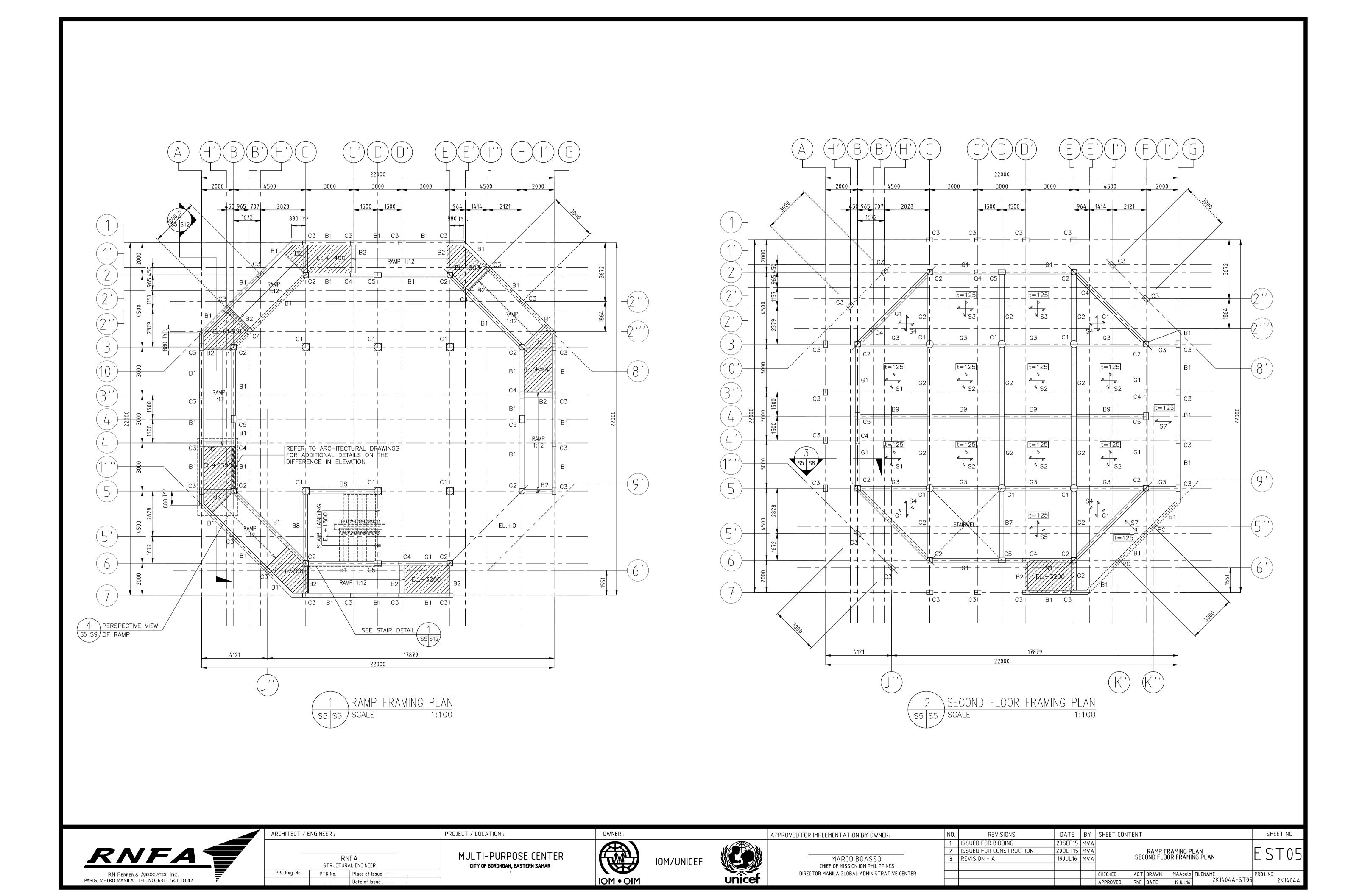


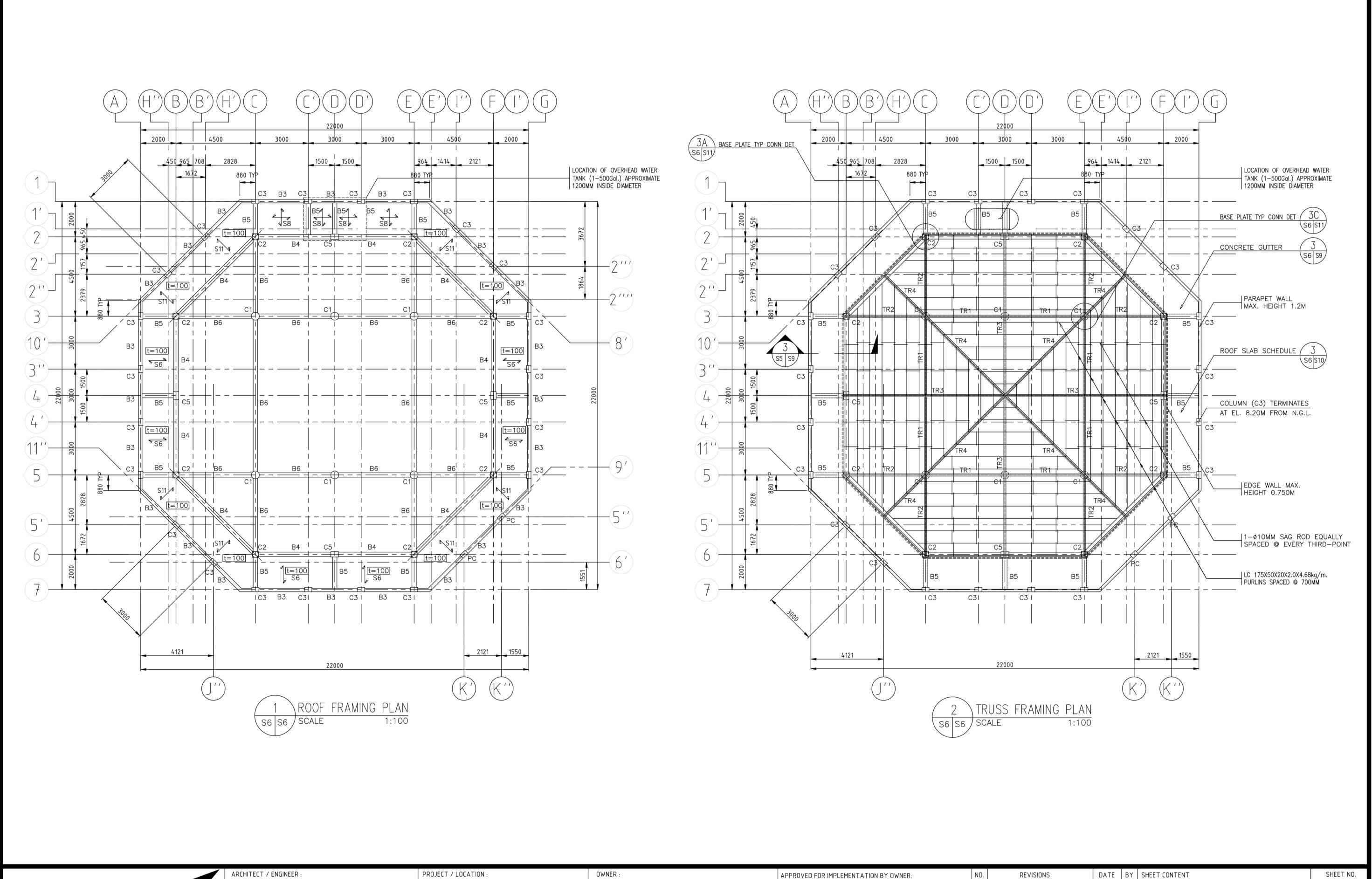




ARCHITECT / ENGINEER PROJECT / LOCATION: OWNER REVISIONS DATE | BY | SHEET CONTENT APPROVED FOR IMPLEMENTATION BY OWNER: TYPICAL TIED COLUMN BAR DETAIL
PIPE SLEEVES DETAIL
TYPICAL BAR BENDING DETAIL
SPLICE LOCATION FOR BEAM REBARS
TYPICAL DROP SLAB DETAIL
TYPICAL WELDED SPLICE DETAIL 1 ISSUED FOR BIDDING 23SEP15 MVA 2 ISSUED FOR CONSTRUCTION 200CT15 MVA MULTI-PURPOSE CENTER RNFA 19 JUL 16 MVA MARCO BOASSO 3 REVISION - A STRUCTURAL ENGINEER **CITY OF BORONGAN, EASTERN SAMAR** CHIEF OF MISSION IOM PHILIPPINES PRC Reg. No. DIRECTOR MANILA GLOBAL ADMINISTRATIVE CENTER Place of Issue : --unicef PTR No. : AQT DRAWN MAApelo FILENAME IOM • OIM 2K1404A-ST3B APPROVED RNF DATE 19JUL16 Date of Issue : ---









RNFA
STRUCTURAL ENGINEER

PRC Reg. No. PTR No.: Place of Issue:--Date of Issue:---



unicef

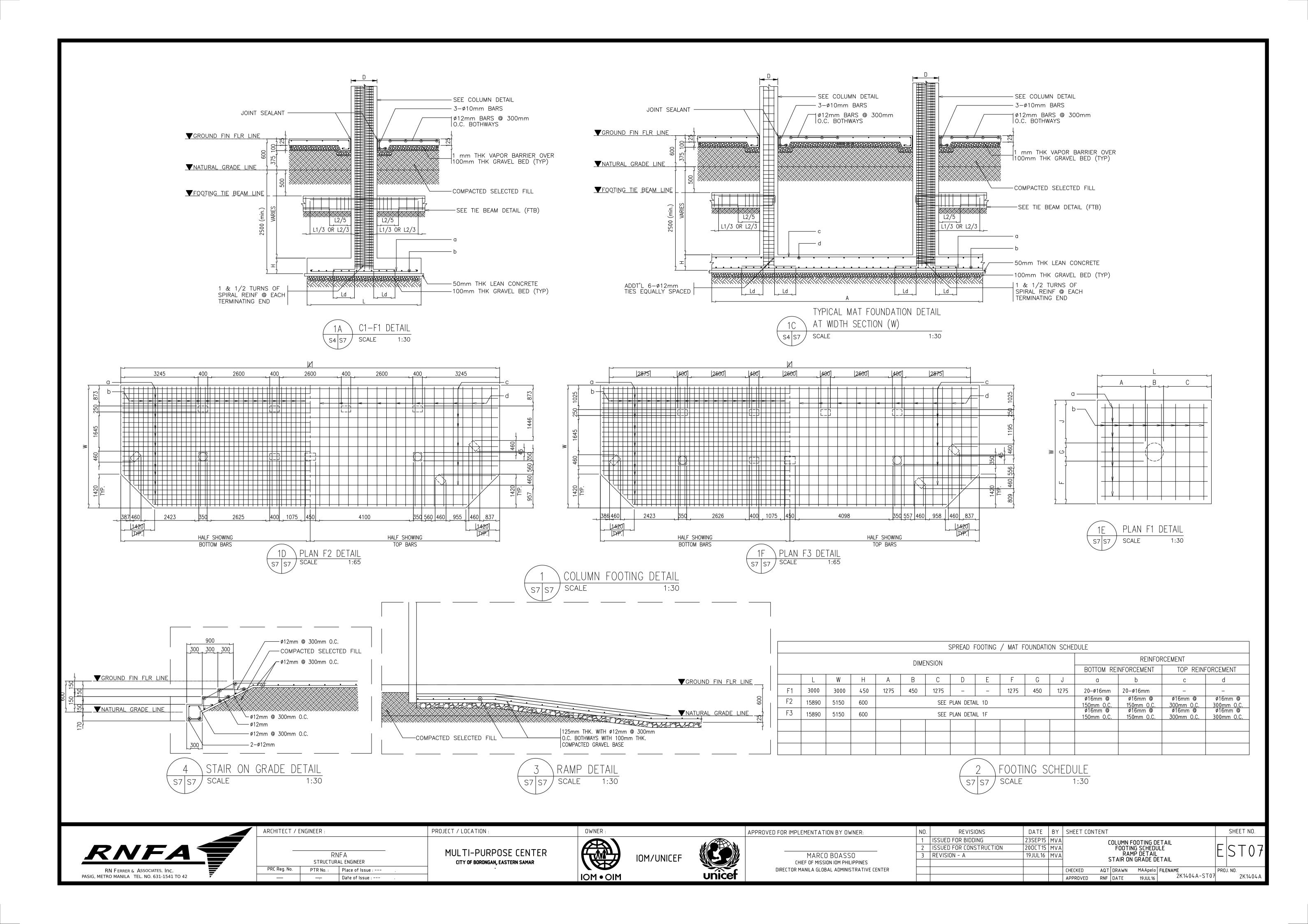
MARCO BOASSO
CHIEF OF MISSION IOM PHILIPPINES
DIRECTOR MANILA GLOBAL ADMINISTRATIVE CENTER

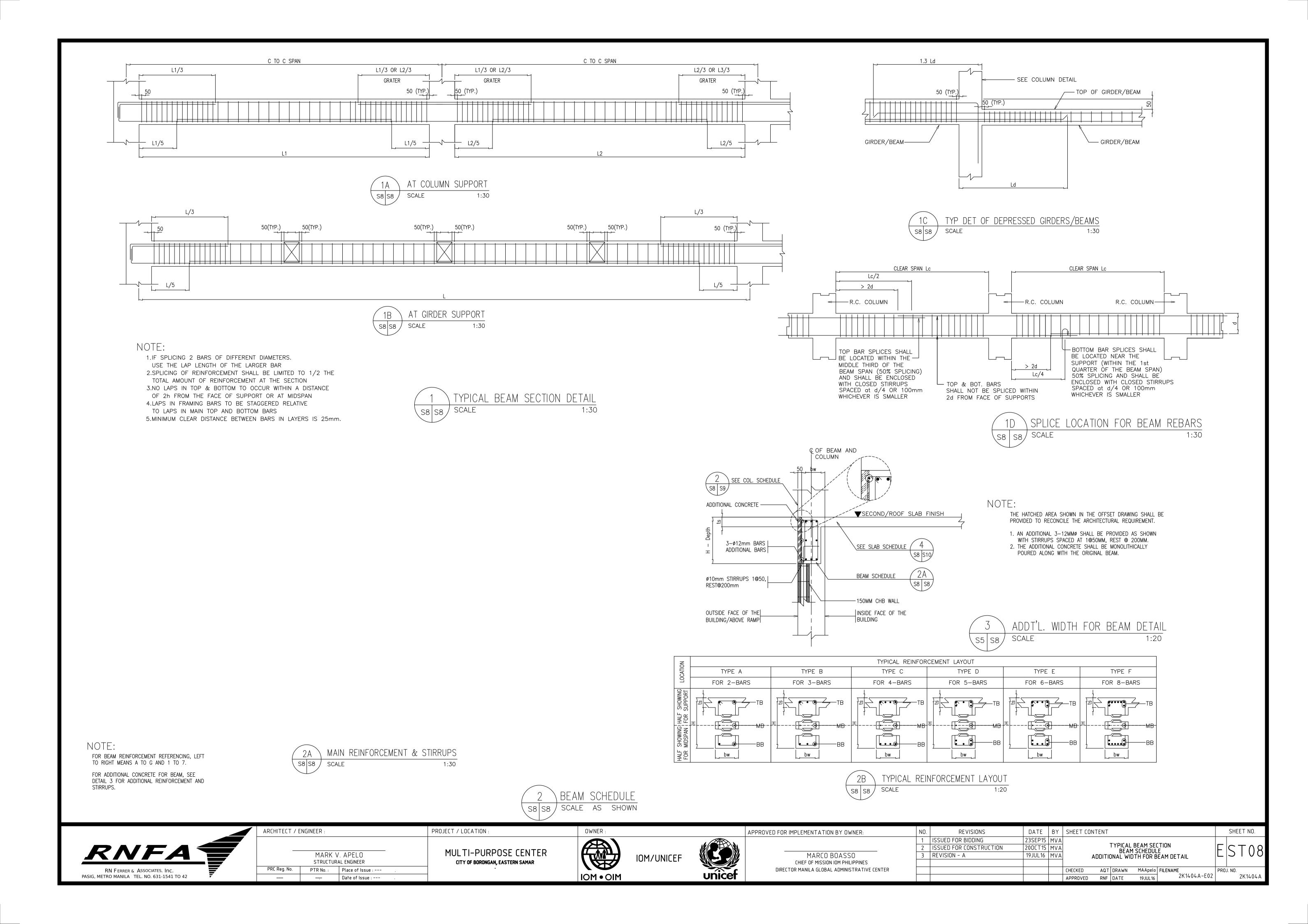
1 ISSUED FOR BIDDING
2 ISSUED FOR CONSTRUCTION
2 OOCT15 MVA
3 REVISION - A
19JUL16 MVA

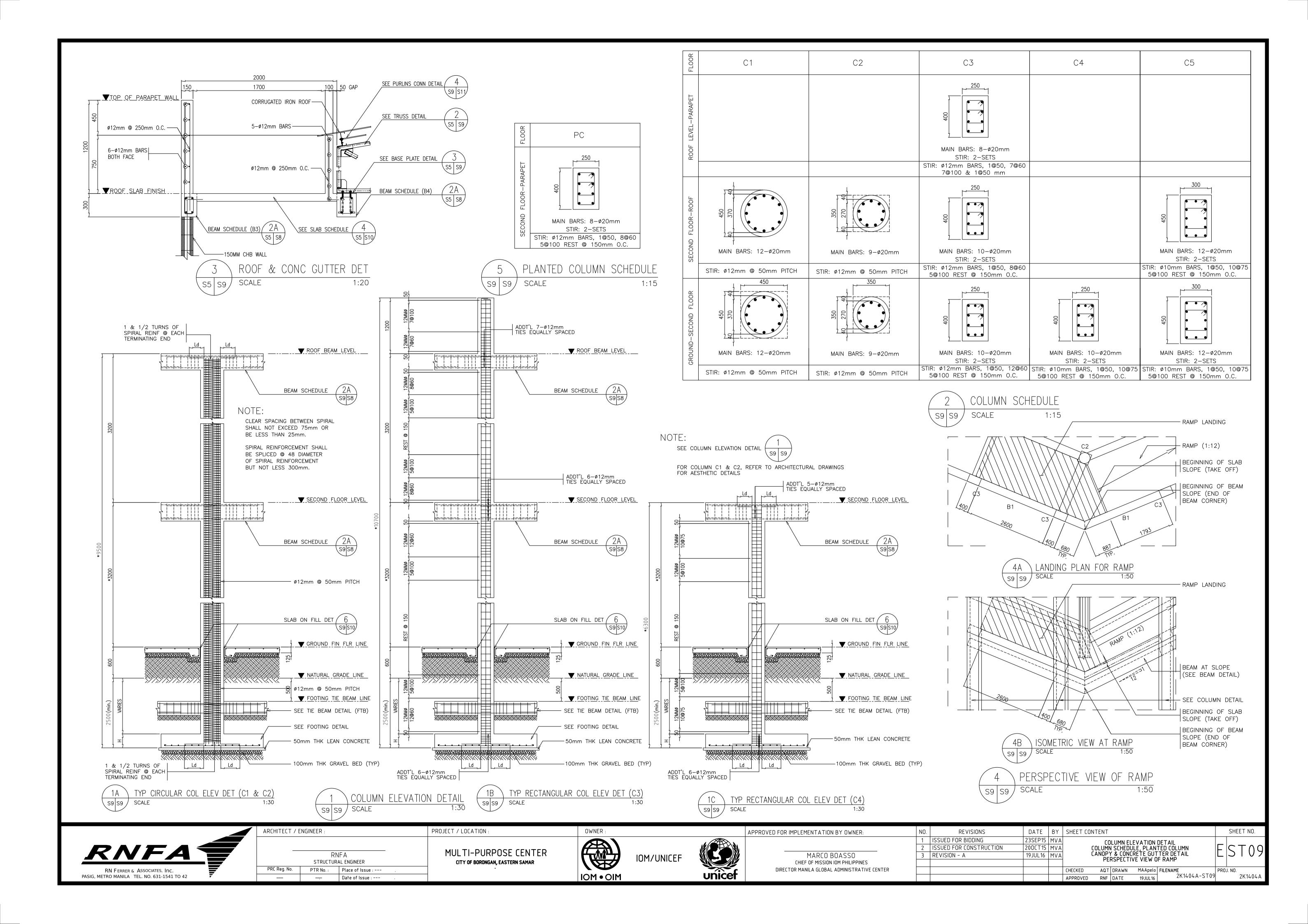
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APPROVED RNF DATE 19JUL16 ZK

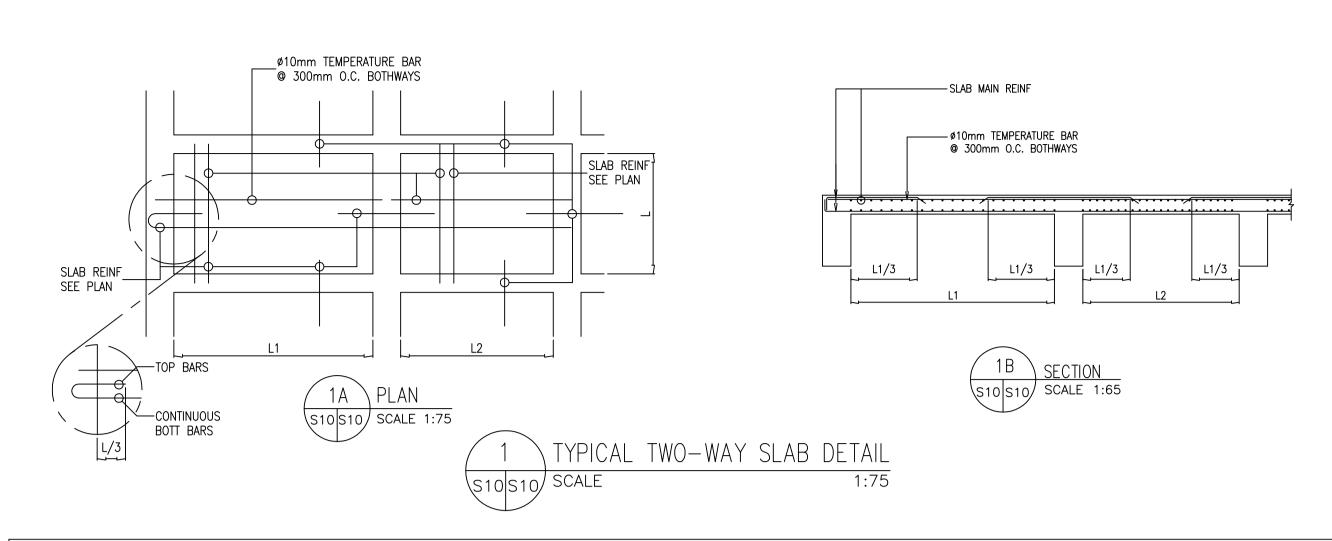
2K1404A-ST06

2K1404A









	SLAB REINFORCEMENT SCHEDULE										
	THOUNECC		SHORT SPAN			LONG SPAN		TEMPERATURE BARS			
MARK	THICKNESS	CONT EDGE	MIDSPAN	DISCONT EDGE	CONT EDGE	MIDSPAN	DISCONT EDGE		REMARKS		
	(mm)	TOP BARS	BOTTOM BARS	TOP BARS	TOP BARS	BOTTOM BARS	TOP BARS				
S1	125	10mmø @ 100mm O.C.	10mmø @ 200mm O.C.	10mmø @ 200mm O.C.	10mmø @ 100mm 0.C.	10mmø @ 200mm 0.C.	_	10mmø @ 300mm O.C. BOTHWAYS	TWO-WAY		
S2	125	10mmø @ 100mm O.C.	10mmø @ 200mm O.C.	_	10mmø @ 100mm 0.C.	10mmø @ 200mm 0.C.	_	10mmø @ 300mm O.C. BOTHWAYS	TWO-WAY		
S3	125	10mmø @ 100mm O.C.	10mmø @ 200mm O.C.	_	10mmø @ 100mm 0.C.	10mmø @ 200mm 0.C.	10mmø @ 200mm 0.C.	10mmø @ 300mm O.C. BOTHWAYS	TWO-WAY		
S4	125	10mmø @ 100mm O.C.	10mmø @ 150mm O.C.	10mmø @ 300mm O.C.	10mmø @ 100mm 0.C.	10mmø @ 150mm 0.C.	10mmø @ 300mm 0.C.	10mmø @ 300mm O.C. BOTHWAYS	TWO-WAY		
S5	125	10mmø @ 100mm O.C.	10mmø @ 200mm O.C.	10mmø @ 200mm O.C.	10mmø @ 100mm 0.C.	10mmø @ 200mm 0.C.	_	10mmø @ 300mm O.C. BOTHWAYS	TWO-WAY		
S8	125	10mmø @ 200mm O.C.	10mmø @ 200mm O.C.	_	_	10mmø @ 200mm 0.C.	10mmø @ 200mm 0.C.	10mmø @ 300mm O.C. BOTHWAYS	TWO-WAY		
S9	125	-	10mmø @ 200mm O.C.	10mmø @ 200mm O.C.	12mmø @ 200mm 0.C.	12mmø @ 200mm 0.C.	_	10mmø @ 300mm O.C. BOTHWAYS	TWO-WAY		

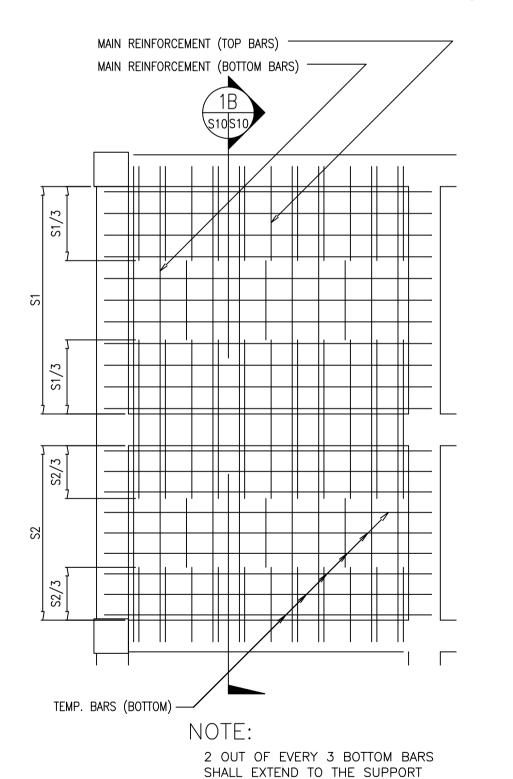
SLAB REINFORCEMENT SCHEDULE (TWO-WAY SLAB) S10S10 SCALE

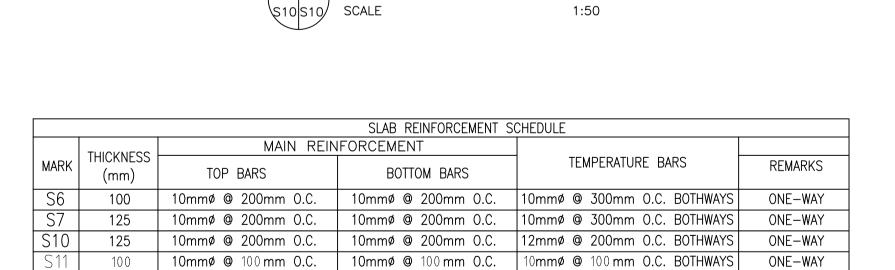
TEMP. BARS (BOTTOM)——

S1/3

S1 = SHORTER SPAN OF SLAB

NOTE:





TYPICAL ONE-WAY SLAB DETAIL

MAIN REINFORCEMENT (TOP BARS) -

MAIN REINFORCEMENT (BOTTOM BARS) —

S2/3

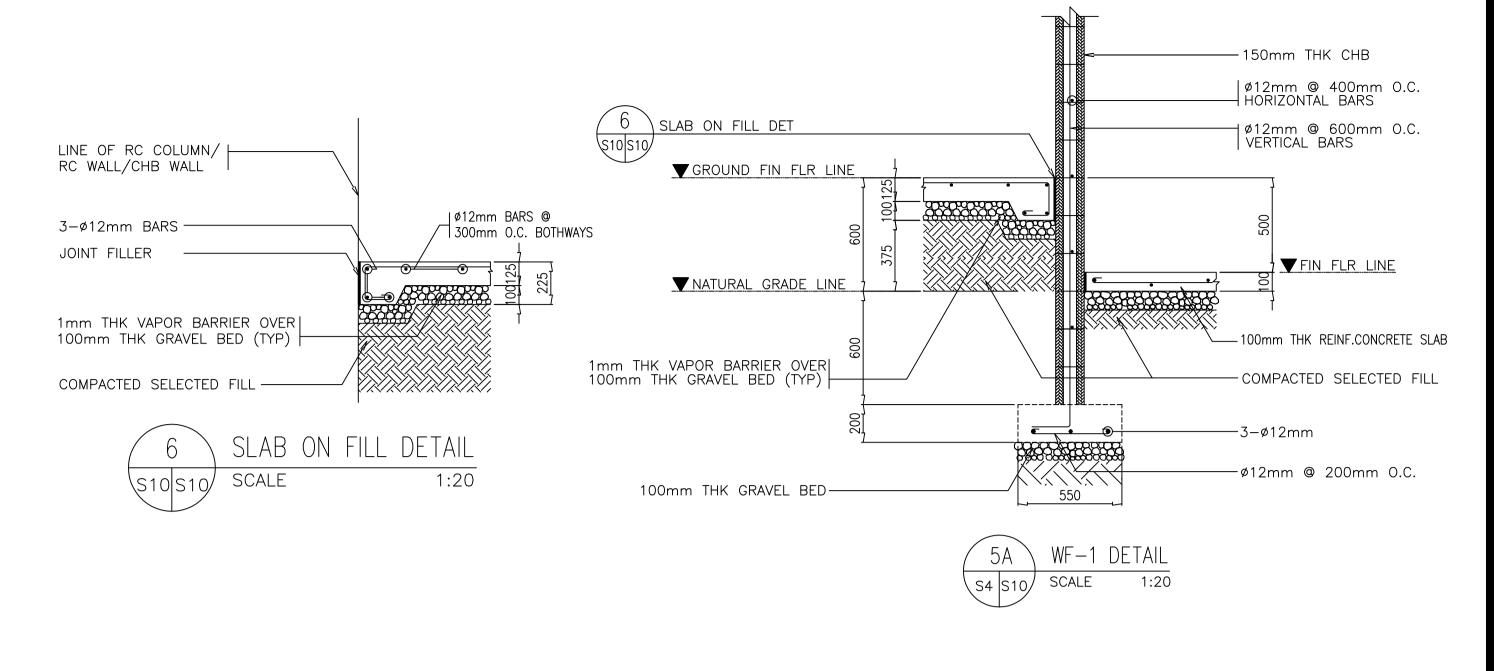
S2/3

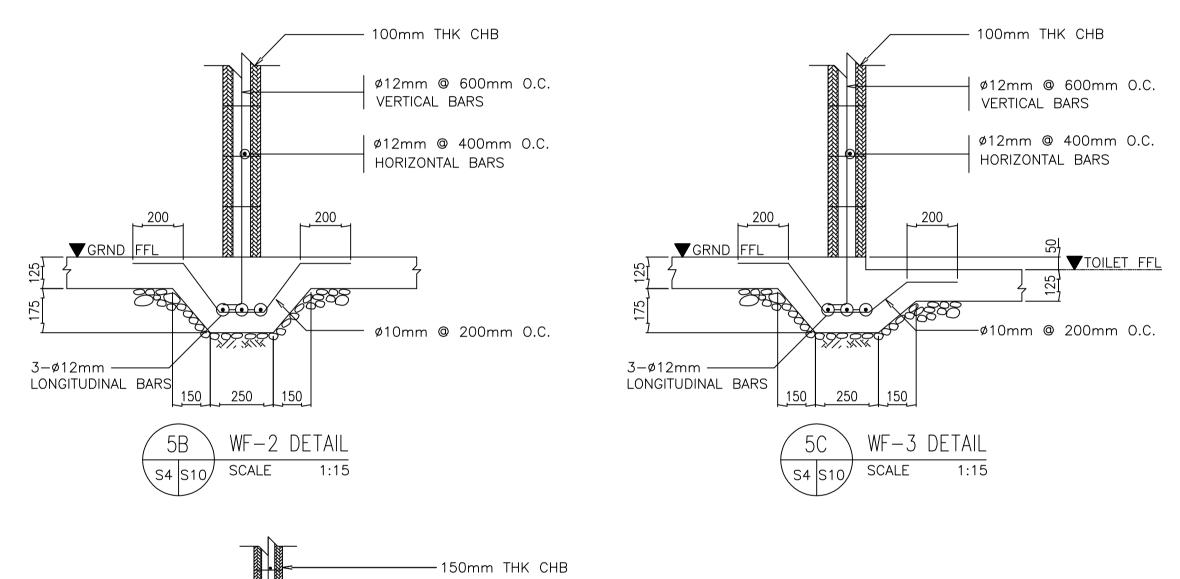
S1/3

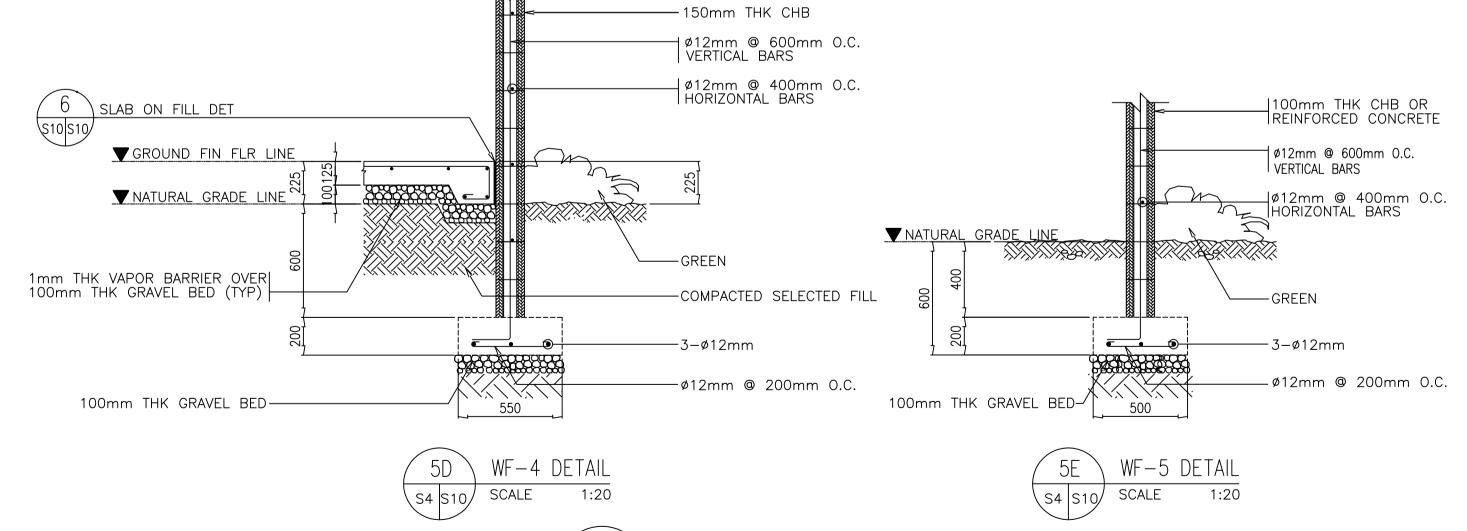
3B SECTION \$10 \$10 SCALE 1:50



PROJECT / LOCATION :







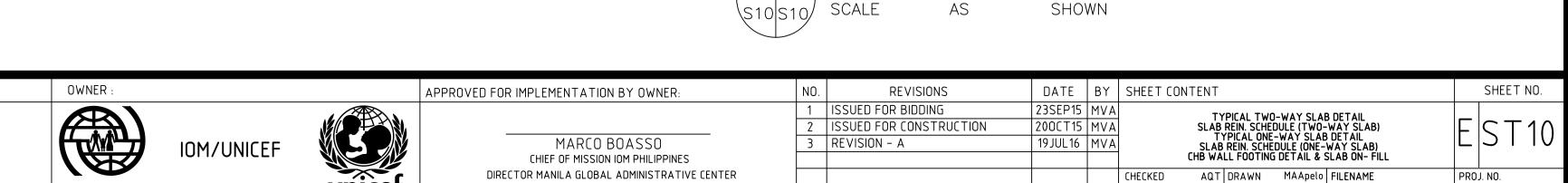
CHB WALL FOOTING DETAIL

2K1404A-ST10

RNF DATE 19JUL16

APPROVED

2K1404A





S10 S10 SCALE 1:65

MULTI-PURPOSE CENTER RNFA STRUCTURAL ENGINEER CITY OF BORONGAN, EASTERN SAMAR PRC Reg. No. PTR No. : Place of Issue : ---Date of Issue : ---

ARCHITECT / ENGINEER :



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